

STATE OF IDAHO
DEPARTMENT OF WATER RESOURCES
BENEFICIAL USE FIELD REPORT

A. GENERAL INFORMATION

Permit No: 72-17016
Exam Date: 09/16/2024

1. Current Owner:
STATE OF IDAHO FISH & GAME COMMISSION
PO BOX 25 BOISE ID 83707-0025

2. **SOURCE:**
GARDEN CREEK

Tributary
SALMON RIVER

Method of Determination: **ArcMap and Field Exam Observation**

B. OVERLAP REVIEW

1. Other water rights with the same place of use: YES Overlap

Water Right No.	Source	Purpose of Use	Basis
72-10256	Garden Creek	Domestic, Stock Water	Decreed
72-10261	Garden Creek	Domestic, Stock Water	Decreed
72-10257	Garden Creek	Irrigation	Decreed

Comments: **Fish Screen is adjacent to Garden Creek and does not interfere with water right uses.**

2. Other water rights with the same point-of-diversion: NO Overlap

Water Right No.	Source	Purpose of Use	Basis
72-10257	Garden Creek	Irrigation	Decreed

The fish screen simply re-routes entrained fish back to the creek without negatively impacting water rights.

Comments: The POD is located very close to the section line and GPS accuracy might make the location somewhat ambiguous. Email correspondence dated August 24, 2022 indicates the applicant and IDWR was aware that future GCDB drift could shift the map location in unpredictable directions and may make the POD location appear to be in an incorrect section.

C. DIVERSION AND DELIVERY SYSTEM1. **LOCATION OF POINT(S) OF DIVERSION:**

GARDEN CREEK NE¼ NE¼, Sec. 31, Twp 14N, Rge 19E, B.M. CUSTER County

Method of Determination: **ArcMap and Field Exam Observation**

PLACE OF USE: FISH PROPAGATION

Twp	Rng	Sec	NE				NW				SW				SE				Totals
			NE	NW	SW	SE	NE	NW	SW	SE	NE	NW	SW	SE	NE	NW	SW	SE	
14N	19E	32						X											

Method of Determination: **ArcMap and Field Exam Observation**

2.

Delivery System Diagram Attached (required). Indicate all major components and distances between components.
☒ Indicate weir size/pipe as applicable.

☒

Map Attached Showing Location(s) of point(s) of diversion and place(s) of use (required). Scale must be 1:24,000 or greater.

☒

Aerial Photo Attached (required for irrigation of 10+ acres).

☒

Photo of Diversion and System Attached

3.

Well or Diversion ID No.*
SGaC-01

D. FLOW MEASUREMENTS

1. Measurements: Head: High water mark of 32 inches (2.667ft). Orifice area: 4-inch diameter hole (.087 ft²)

E. FLOW CALCULATIONS

Measured Method: A flow measurement was obtained using an orifice flow equation discussed in an August 24, 2011 Memorandum that utilizes a head measurement and the area of an orifice.

$$Q = C_d A_o (2gh)^{1/2}$$

Where: C_d = coefficient of discharge for orifice

A_o = Area of orifice opening in ft²

$G = 32.2$ ft per second²

H = measured head above center of orifice opening (in feet)

$$0.67(0.087 \text{ ft}^2) [2(32.2 \text{ ft per second}^2)(2.667 \text{ ft})]^{1/2} = 0.7614 \text{ or } \mathbf{0.76 \text{ cfs}}$$

F. VOLUME CALCULATIONS

1. Volume Calculations for irrigation: **N/A**

2. Volume Calculations for Other Uses: The water right diverted through fish screen is authorized for a 214-day irrigation season.

$$V_{DR} = 0.76 \text{ cfs} \times 214 \text{ days} \times 1.9835 = \mathbf{322.6 \text{ AF}}$$

G. NARRATIVE/REMARKS/COMMENTS

Permit 72-17016 was filed on July 29, 2022 by the Idaho Department of Fish and Game for the diversion of 0.80 cfs of non-consumptive use of water for a fish bypass screen structure. The fish bypass structure and the Point of Diversion (POD) are both located on land owned by Yacomella Revocable Living Trust & Tony Yacomella. The structure itself, is located between the Point of Diversion and the Place of Use (POU) of the Decreed water right authorized to use the POD. A bypass pipe is used to flush any entrained fish back to Garden Creek.

On 09/16/2024 I performed the field exam. The POD on Garden Creek consists of a lockable headgate that diverts water down a side channel to the fish screen structure. The POD is located very close to the section line and GPS field accuracy and GCDB drift might make the map location somewhat ambiguous. Email correspondence dated August 24, 2022, indicates the applicant and IDWR was aware that future GCDB drift could shift the map location unpredictably and may make the POD location appear to be in an incorrect section. Because of the GCDB uncertainty the POD location will not be changed based on this field exam. The POD and the POU will be linked to IDWR maps as authorized on the approved permit.

Water from the POD flows through the fish screen structure for downstream beneficial uses. A small portion of water carries any entrained fish from the structure back to Garden Creek through a buried 10-inch bypass pipe. Water flow through the bypass pipe is controlled by a 4-inch diameter orifice (0.087 ft^2) cut in a 16-inch square slide gate. On the day of the field exam a low flow of water was passing through the structure. An obvious high-water mark was visible on the side walls of the structure. A maximum flow rate was determined by using the area of the orifice and the height of the high-water mark (from the center of the orifice opening). The flow rate and volume calculations are shown above in Sections E and F.

Conditions on the permit have been complied with and I recommend licensing Water Permit 72-17016 for a flow rate of 0.76 cfs and an annual volume of 322.6 AF.

Have conditions of permit approval been met? X Yes No

H. RECOMMENDATIONS**1. Recommended Amounts**

<u>Beneficial Use</u>	<u>Period of Use</u>	<u>Rate of Diversion</u>	<u>Annual Volume</u>
FISH PROPAGATION	04/01 to 10/31	0.76 CFS	322.6 AF
<u>Totals:</u>		0.76 CFS	322.6 AF

2. Recommended Amendments

☐ Change P.D. as reflected above ☐ Add P.D. as reflected above ☒ None

☐ Change P.U. as reflected above ☐ Add P.U. as reflected above ☒ None

I. AUTHENTICATION

Kent Aasa - Water Resource Agent, Senior

Field Examiner's Name

Date

10-25-2024

Reviewer

Date

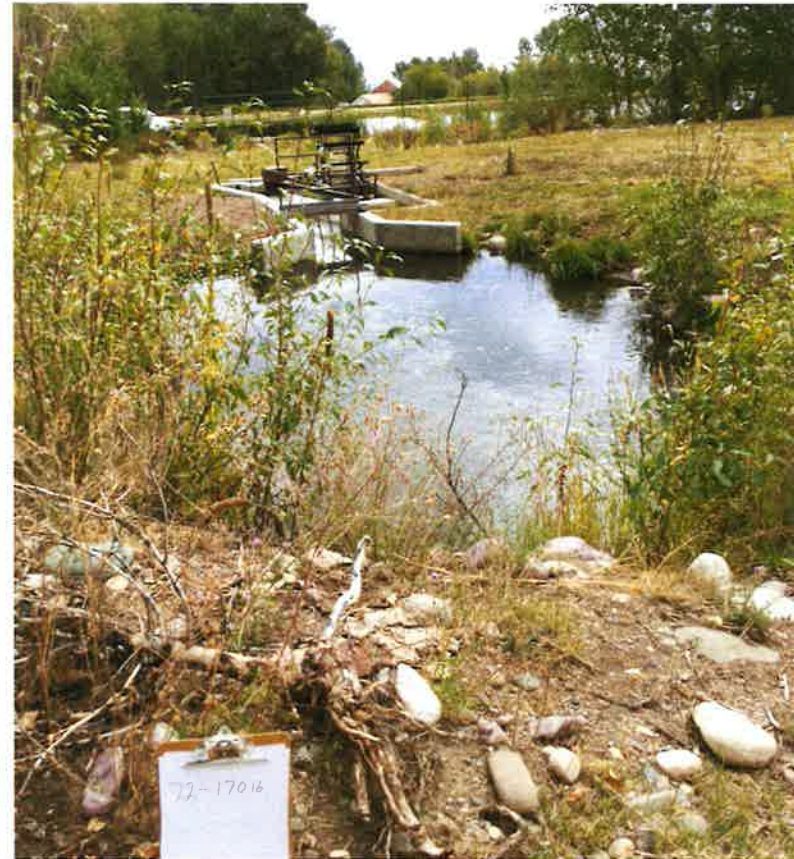
11/29/2024

72-17016 POD



72-17016 POU

© 44°30'18.31", -114°15'11.16" ±16ft ▲ 5435ft



72-17016 Bypass Pipe

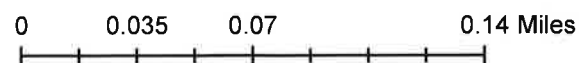


Field Exam 72-17016

72-17016 SYSTEM DIAGRAM



72-17016
FISH PROPAGATION



-  Place Of Use Boundary
-  Point of Diversion
-  State Outline
-  Townships
-  PLS Sections
-  Quarter Quarters



MEMORANDUM

TO: Basin 71 and 72 IDF&G Fish Bypass Permit Files

FROM: Corey Skinner

DATE: August 24, 2011

SUBJECT: Methodology for Flow Measurements

Recently Benny Walker and I conducted several licensing exams for IDF&G bypass permits in Basins 71 and 72. In general, these permits are for an amount of non consumptive flow that is diverted from a stream through existing diversions. The diverted water flows down a ditch, into a fish screen structure, and then flows through a bypass pipe back to the original stream source at a point downstream of the original diversion. The purpose of this flow is to flush fish back to the stream that may otherwise be trapped in the ditch. The fish screen structures include a slide gate plate with an opening that allows water to flow through the plate into the bypass pipe back to the stream. Due to the variability in diverted flows, difficulty in measuring flows for certain structures due to the way they were constructed, distance between structures, and other variables a methodology is needed to estimate maximum flows that are diverted and utilized by the IDF&G bypass permits.

After examining several of the structures and bypass systems and examining several different sources of water measurement literature, it was determined that the most appropriate method would be to utilize an orifice flow equation that is typically used to estimate flow through orifice openings in tanks. Attachment "A" discusses the equation, and the associated orifice coefficient, in particular C_d (coefficient of discharge). The equation is as follows...

$$Q = C_d A_o (2gh)^{1/2}$$

Where C_d = coefficient of discharge for orifice

A_o = Area of orifice opening in feet²

g = 32.2 ft per second²

h = measured head above center of orifice opening (in feet)

All variables can be measured in the field except for C_d (coefficient of discharge). Based on water measurement literature, this particular coefficient can vary from numbers around 0.6 to approximately 1.0 as shown in Table 3.10 in "Attachment A". In addition, the exact geometry of the opening (orifice) in the various slide gate plates that Benny and I observed is not shown in Table 3.10 in "Attachment A". Examination of various sources of water measurement literature and websites on the internet did not reveal much more in the way of appropriate C_d (coefficient of discharge) values. One reputable published source indicated that " C_d values can vary, but are generally around 0.70".

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Basin 71 and 72 IDF&G Fish Bypass Permits Memo

August 24, 2011

Due to the uncertainty in the possible C_d values, but considering the similarity in the opening (orifice) in the various slide gate plates and the overall bypass systems that Benny and I observed, it was determined that if an accurate measurement of the actual flow through a bypass system could be obtained, and using the orifice area and head reading at the time of measurement, the orifice discharge equation could be used to obtain an actual C_d (coefficient of discharge) value for the openings (orifices) in the various slide gate plates Benny and I were observing. This calculated C_d (coefficient of discharge) value could then be used in the same equation, for other similar bypass systems, and would result in more accurate calculated results. In addition, the equation could be utilized to estimate maximum flows through the bypass systems by measuring high water marks on the fish screen structures even if very little, or even no, flow was flowing through the bypass at the time of the exam.

Yesterday, Benny and I found a fish screen bypass system that we felt could be accurately measured and was very typical of the various systems. This system was located on Fourth of July Creek, and happened to be one of the permits, 71-10909, that needed to be examined for licensing. A Sparling Meter, mounted in a pipe section (State of Idaho ID# 4335) that could be mounted on the bypass discharge pipe was utilized to obtain flow measurements at a given head reading. Two separate, timed, measurements were conducted with both measurements resulting in measured flows of 0.68 cfs.. The following photo shows one of these measurements being conducted...



Using this measured flow, the orifice discharge equation, an orifice measurement, and head measurements, finds that the C_d (coefficient of discharge) is calculated to be 0.67 as shown below...

$$Q = C_d A_o (2gh)^{1/2}$$

Where C_d = coefficient of discharge for orifice
 A_o = 0.087 feet² (based on measured 4 inch diameter orifice opening)
 g = 32.2 ft per second²
 h = 2.125 feet (measured at 25.5 inches)

$$0.68 \text{ cfs} = C_d (0.087 \text{ ft}^2) [2(32.2 \text{ ft per second}^2)(2.125 \text{ ft})]^{1/2}$$

$$C_d = 0.67$$

It is appropriate to use a C_d (coefficient of discharge) value of 0.67 and the orifice discharge equation to estimate the maximum flow for the permits for IDF&G bypass permits in Basins 71 and 72.

Gradual, $\phi < 22^\circ$ Sudden, $\phi > 22^\circ$

Enlargement	$2.6(\sin \phi)(1-\beta^2)^2$	$(1-\beta^2)^2$	3.81
Contraction	$0.8(\sin \phi)(1-\beta^2)$	$\frac{1}{2}(1-\beta^2)\sqrt{\sin \phi}$	3.82

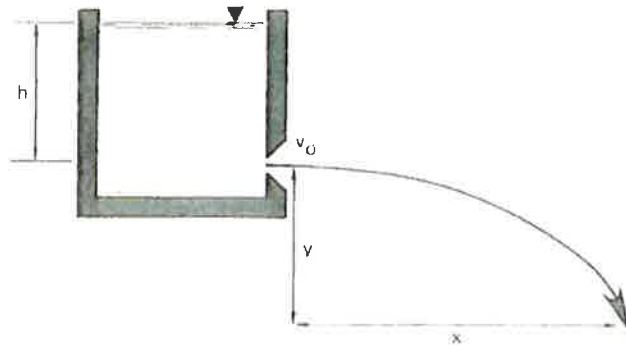


Figure 3.14 Discharge From a Tank

4 HEAD ADDITIONS/EXTRACTIONS

A pump adds head (energy) to the fluid stream. A turbine extracts head from the fluid stream. The amount of head added or extracted can be found by evaluating Bernoulli's equation (equation 3.55) on both sides of the device.

$$h_A = H_2 - H_1 \quad (\text{pumps}) \quad 3.83$$

$$h_E = H_1 - H_2 \quad (\text{turbines}) \quad 3.84$$

The head increase from a pump is given by equation 3.85.

$$h_A = \frac{(550)(\text{pump input horsepower})\eta_{\text{pump}}}{\dot{m}} \left(\frac{g_c}{g} \right) \quad 3.85$$

Bernoulli's equation also can be used to calculate the power available to a turbine in a fluid stream by multiplying the total energy by the mass flow rate. This is called the *water horsepower*.

$$P = \dot{m}H \left(\frac{g}{g_c} \right) = \dot{m} \left(\frac{p}{\gamma} + \frac{v^2}{2g} + z \right) \left(\frac{g}{g_c} \right) \quad 3.86$$

$$\dot{m} = \rho A v \quad 3.87$$

$$\text{whp} = \frac{P}{550} \quad 3.88$$

Pumps and turbines are covered in greater detail in chapter 4.

5. DISCHARGE FROM TANKS

Flow from a tank discharging liquid to the atmosphere through an opening in the tank wall (figure 3.14) is affected by both the area and the shape of the opening. At the orifice, the total head of the fluid is converted into kinetic energy according to equation 3.89.⁶

$$v_o = C_v \sqrt{2gh} \quad 3.89$$

⁶ Although the term g_c appears in the equation for velocity head (equation 3.53), here it is the local gravity, g , which appears in equation 3.89. An analysis of equation 3.197 will show you why this is so.

C_v is the *coefficient of velocity* which can be calculated from the *coefficients of discharge and contraction*. Typical values of C_d , C_c , and C_v are given in table 3.10.

$$C_v = \frac{C_d}{C_c} \quad 3.90$$

The discharge from the orifice is

$$\begin{aligned} \dot{V} &= (C_c A_o) v_o = C_c A_o C_v \sqrt{2gh} \\ &= C_d A_o \sqrt{2gh} \end{aligned} \quad 3.91$$

The head loss due to turbulence at the orifice is

$$h_f = \left(\frac{1}{C_v^2} - 1 \right) \frac{v_o^2}{2g} \quad 3.92$$

The discharge stream coordinates (see figure 3.14) are

$$x = v_o t = v_o \sqrt{\frac{2y}{g}} = 2C_v \sqrt{hy} \quad 3.93$$

$$y = \frac{gt^2}{2} = \frac{g}{2} \left(\frac{x}{v_o} \right)^2 \quad 3.94$$

The fluid velocity at a point downstream of the orifice is

$$v_x = v_o \quad 3.95$$

$$v_y = gt \quad 3.96$$

If the liquid in a tank is not being replenished constantly, the static head forcing discharge through the orifice will decrease. For a tank with a constant cross-sectional area, the time required to lower the fluid level from level h_1 to h_2 is calculated from equation 3.97.

$$t = \frac{2A_t(\sqrt{h_1} - \sqrt{h_2})}{C_d A_o \sqrt{2g}} \quad 3.97$$

If the tank has a varying cross section, the following basic relationship holds.

$$Q dt = -A_t dh \quad 3.98$$

Table 3.10
Orifice Coefficients for Water
(fully turbulent)

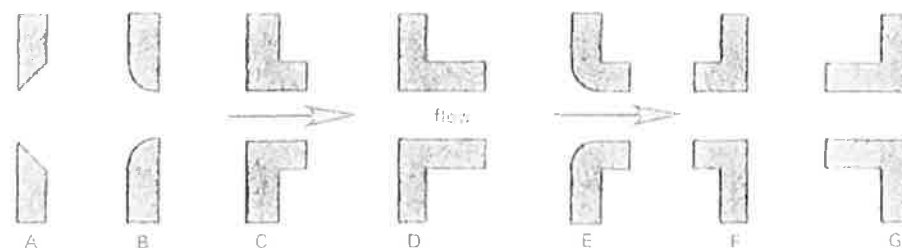


illustration	description	C_d	C_v	C_c
A	sharp-edged	0.62	0.63	0.95
B	round-edged	0.98	1.00	0.98
C	short tube (fluid separates from walls)	0.61	1.00	0.61
D	short tube (no separation)	0.82	1.00	0.82
E	short tube with rounded entrance	0.97	0.99	0.96
F	reentrant tube, length less than one-half of pipe diameter	0.54	0.55	0.99
G	reentrant tube, length 2 to 3 pipe diameters	0.72	1.00	0.72
not shown	smooth, well-tapered nozzle	0.98	0.99	0.99

An expression for the tank area, A_t , as a function of h , must be determined. Then, the time to empty the tank from height h_1 to lower height h_2 is

$$t = \int_{h_1}^{h_2} \frac{A_t dh}{C_d A_o \sqrt{2gh}} \quad 3.99$$

For a tank being fed at a rate, $\dot{V}_m$, which is less than the discharge through the orifice, the time to empty expression is

$$t = \int_{h_1}^{h_2} \frac{A_t dh}{(C_d A_o \sqrt{2gh}) - \dot{V}_m} \quad 3.100$$

When a tank is being fed at a rate greater than the discharge, equation 3.100 will become positive, indicating a rising head. t then will be the time it takes to raise the fluid level from h_1 to h_2 .

The preceding discussion has assumed that the tank has been open or vented to the atmosphere. If the fluid is discharging from a pressurized tank, the total head will be increased by the gage pressure converted to head of fluid by means of equation 3.52.

Example 3.22

A 15' diameter tank discharges 150 F water through a sharp edged 1" diameter orifice. If the original water depth is 12' and the tank is continually pressurized to 50 psig, find the time to empty the tank.

At 150 F, $\rho = 61.20 \text{ lbm/ft}^3$

For the orifice,

$$A_o = 0.00545 \text{ ft}^2, \quad C_d = 0.62$$

$$h_1 = 12 + \frac{(50)(1.11)}{61.2} = 129.65 \text{ ft}$$

$$h_2 = \frac{(50)(1.41)}{61.20} = 117.65 \text{ ft}$$

From equation 3.97,

$$t = \frac{2[\pi(7.5)^2](\sqrt{129.65} - \sqrt{117.65})}{(0.62)(0.00545)\sqrt{(2)(32.2)}} = 7035 \text{ seconds}$$

6 CULVERTS AND SIPHONS

A culvert is a water path used to drain runoff from an obstructing geographical feature. Most culvert designs are empirical. However, if the entrance and exit of the culvert both are submerged, the discharge will be independent of the barrel slope. (Chapter 5 treats this subject in greater detail.) In that case, equation 3.101 can be used to evaluate the discharge. h is the difference in surface levels of the headwater and tailwater.

$$\dot{V} = C_d A \sqrt{2gh} \quad 3.101$$