

STATE OF IDAHO
DEPARTMENT OF WATER RESOURCES
BENEFICIAL USE FIELD REPORT

A. GENERAL INFORMATION

Permit No: 74-15845

Exam Date: 5/22/2012

1. Current Owner: STATE OF IDAHO
DEPT OF FISH & GAME
PO BOX 25
BOISE ID 83707
(208)287-2714

2. Accompanied by: Larry Weeks

Phone No: (208) 756-6022Address: Region 7, PO Box 1336, Salmon, ID 83647Relationship to Permit Holder: Easement Specialist3. **SOURCE**

BOHANNON CREEK

TRIBUTARY

LEMHI RIVER

B. OVERLAP REVIEW

1. Other water rights with the same place of use: NONE
2. Other water rights with the same point of diversion: 74-185, 74-186, and 74-723

C. DIVERSION AND DELIVERY SYSTEM1. **LOCATION OF POINT(S) OF DIVERSION:**

BOHANNON CREEK SW1/4NE1/4 Sec. 22, Twp 21N, Rge 23E, B.M. LEMHI County

Method of Determination: Visual Inspection and Existing Spatial Data information

2.

PLACE OF USE: FISH PROPAGATION

Twp	Rge	Sec	NE				NW				SW				SE				Totals
			<u>NE</u>	<u>NW</u>	<u>SW</u>	<u>SE</u>	<u>NE</u>	<u>NW</u>	<u>SW</u>	<u>SE</u>	<u>NE</u>	<u>NW</u>	<u>SW</u>	<u>SE</u>	<u>NE</u>	<u>NW</u>	<u>SW</u>	<u>SE</u>	
21N	23E	22														X			

Method of Determination: Visual Inspection and GIS

3. YES Delivery System Diagram Attached (required). Indicate all major components and distances between components. Indicate weir size/pipe as applicable.
- YES Aerial Photo Attached (required for irrigation of 10+ acres).
- YES Photo of Diversion and System Attached

4.

Well or Diversion Identification No.*	Motor Make	Hp	Motor Serial No.	Pump Make	Pump Serial No. or Discharge Size
Site Tag # A0018565	NONE				Open Ditch

*Code to correspond with No. on map and aerial photo

D. FLOW MEASUREMENTS

1.

Measurement Equipment	Type	Make	Model No.	Serial No.	Size	Calib. Date

2. Measurements: The system was set up with a standard orifice that is used on all of the fish screens in the Salmon River and Lemhi River area. This orifice has been checked for accuracy and found to be reliable. Please see Cory Skinner's memo dated 8/24/2011.

E. FLOW CALCULATIONS X Additional Computation Sheets Attached

Measured Method:

Measurement = 0.92 cfs
Permit allowed = 0.80 cfs

License recommend for 0.80 cfs.

F. VOLUME CALCULATIONS

1. Volume Calculations for Irrigation:

$$V_{I.R.} = (\text{Acres Irrigated}) \times (\text{Irrigation Requirement}) =$$

$$V_{D.R.} = [\text{Diversion Rate (cfs)}] \times (\text{Days in Irrigation season}) \times 1.9835 =$$

$$V = \text{Smaller of } V_{I.R.} \text{ and } V_{D.R.} =$$

2. Volume Calculations for Other Uses:

0.80 cfs X 246 days in irrigation season X 1.9835 = 390.4 af of use per year.

G. NARRATIVE/REMARKS/COMMENTS

Water from Ditch is diverted from Bohannon Creek through a ditch that travels approximately 600 feet southwest to the fish screen. At the fish screen the majority of the water continues down the ditch to supply water to the other water rights using this diversion. A maximum of 0.92 cfs is diverted through a 10 inch pipe back to the creek to prevent fish kills in the creek. The water going back to the creek is run through a 4 inch orifice for measurement purposes. The Spatial data point names this point of diversion as BO12, but Fish and Game describe it as LBC-05.

The fish screen was actually located in the NWSE instead of the SWNE as listed on the permit. A licensing amendment has been sought to correct this error.

Have conditions of permit approval been met? ☒ Yes ☐ No

H. RECOMMENDATIONS

1. Recommended Amounts

<u>BENEFICIAL USE</u>	<u>PERIOD OF USE</u>	<u>DIVERSION RATE</u>	<u>ANNUAL VOLUME</u>
FISH PROPAGATION	04/01 to 11/01	0.800 CFS	390.40 AF
	<u>Totals:</u>	0.800 CFS	390.40 AF

2. Recommended Amendments

☐ Change P.D. as reflected above ☐ Add P.D. as reflected above ☐ None
☒ Change P.U. as reflected above ☐ Add P.U. as reflected above ☐ Other

I. AUTHENTICATION Daniel A. Nelson – Staff Hydrologist

Field Examiner's Name Daniel A. Nelson Date 11-6-2012

Reviewer _____ Date _____

State of Idaho
Department of Water Resources
Field Exam Map
74-15845

This map depicts the FISH PROPAGATION place of use boundary for this water right at the time of this approval and is attached to the approval document solely for illustrative purposes.



0 0.0275 0.055 0.11 Miles

-  Point of Diversion
-  Place Of Use Boundary
-  Townships
-  PLS Sections
-  Quarter Quarters



Field Exam Flow Rate Calculation Sheet for Fish Screen Orifices

Water Right Number: 74-7337
Field Examiner: Daniel Nelson
Date of Exam: 5/21/2012

Discharge from a tank through an orifice (see attached memo from water right 72-16754 by Corey Skinner for a formula and orifice coefficient.

Formula = $C_d A_o (2gh)^{1/2}$

Where C_d = coefficient of discharge
 A_o = Area of orifice opening in Square feet
 $g=32.2$ feet per second²
 h = Measured head above center of orifice opening in feet

Orifice Diameter in inches =	4
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$$A_o = (\pi r^2) / 144 = \text{Square Feet}$$

A_o = 0.087

Head above center of orifice in Inches =	46
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 (Measured to High Water Mark)

h = 3.833

Coefficient of Discharge for Orifice* =	0.67
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Total Diversion Rate = 0.92

* Per Cory Skinner's memo for Basin 71 and 72 IDF&G Fish Bypass Permit Files dated August 24, 2011, the Coefficient for round edge orifice should be 0.67.

	Gradual, $\phi < 22^\circ$	Sudden, $\phi > 22^\circ$	
Enlargement	$2.6(\sin \phi)(1 - \beta^2)^2$	$(1 - \beta^2)^2$	3.81
Contraction	$0.8(\sin \phi)(1 - \beta^2)$	$\frac{1}{2}(1 - \beta^2)\sqrt{\sin \phi}$	3.82

4 HEAD ADDITIONS/EXTRACTIONS

A pump adds head (energy) to the fluid stream. A turbine extracts head from the fluid stream. The amount of head added or extracted can be found by evaluating Bernoulli's equation (equation 3.55) on both sides of the device.

$$h_A = H_2 - H_1 \quad (\text{pumps}) \quad 3.83$$

$$h_E = H_1 - H_2 \quad (\text{turbines}) \quad 3.84$$

The head increase from a pump is given by equation 3.85.

$$h_A = \frac{(550)(\text{pump input horsepower})\eta_{\text{pump}}}{\dot{m}} \left(\frac{g_c}{g} \right) \quad 3.85$$

Bernoulli's equation also can be used to calculate the power available to a turbine in a fluid stream by multiplying the total energy by the mass flow rate. This is called the *water horsepower*.

$$P = \dot{m}H \left(\frac{g}{g_c} \right) = \dot{m} \left(\frac{p}{\gamma} + \frac{v^2}{2g} + z \right) \left(\frac{g}{g_c} \right) \quad 3.86$$

$$\dot{m} = \rho A v \quad 3.87$$

$$\text{whp} = \frac{P}{550} \quad 3.88$$

Pumps and turbines are covered in greater detail in chapter 4.

5. DISCHARGE FROM TANKS

Flow from a tank discharging liquid to the atmosphere through an opening in the tank wall (figure 3.14) is affected by both the area and the shape of the opening. At the orifice, the total head of the fluid is converted into kinetic energy according to equation 3.89.⁶

$$v_o = C_v \sqrt{2gh} \quad 3.89$$

⁶ Although the term g_c appears in the equation for velocity head (equation 3.53), here it is the local gravity, g , which appears in equation 3.89. An analysis of equation 3.197 will show you why this is so.

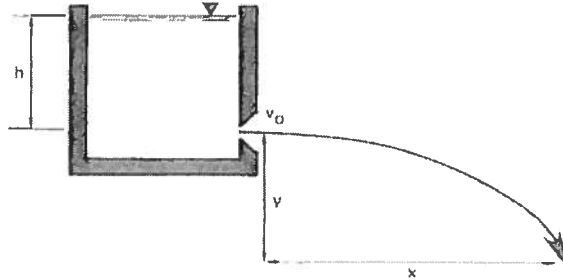


Figure 3.14 Discharge From a Tank

C_v is the *coefficient of velocity* which can be calculated from the *coefficients of discharge* and *contraction*. Typical values of C_v , C_d , and C_c are given in table 3.10.

$$C_v = \frac{C_d}{C_c} \quad 3.90$$

The discharge from the orifice is

$$\begin{aligned} \dot{V} &= (C_c A_o) v_o = C_c A_o C_v \sqrt{2gh} \\ &= C_d A_o \sqrt{2gh} \end{aligned} \quad 3.91$$

The head loss due to turbulence at the orifice is

$$h_f = \left(\frac{1}{C_v^2} - 1 \right) \frac{v_o^2}{2g} \quad 3.92$$

The discharge stream coordinates (see figure 3.14) are

$$x = v_o t = v_o \sqrt{\frac{2y}{g}} = 2C_v \sqrt{hy} \quad 3.93$$

$$y = \frac{gt^2}{2} = \frac{g}{2} \left(\frac{x}{v_o} \right)^2 \quad 3.94$$

The fluid velocity at a point downstream of the orifice is

$$v_x = v_o \quad 3.95$$

$$v_y = gt \quad 3.96$$

If the liquid in a tank is not being replenished constantly, the static head forcing discharge through the orifice will decrease. For a tank with a constant cross-sectional area, the time required to lower the fluid level from level h_1 to h_2 is calculated from equation 3.97.

$$t = \frac{2A_t(\sqrt{h_1} - \sqrt{h_2})}{C_d A_o \sqrt{2g}} \quad 3.97$$

If the tank has a varying cross section, the following basic relationship holds.

$$Q dt = -A_t dh \quad 3.98$$

Table 3.10
Orifice Coefficients for Water
(fully turbulent)

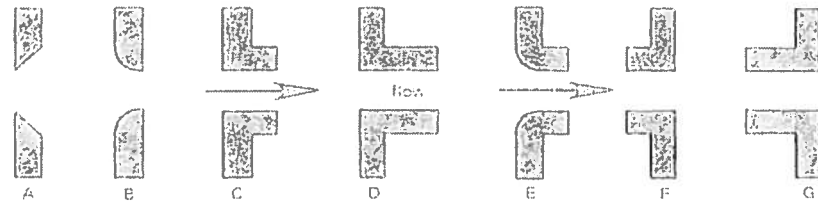
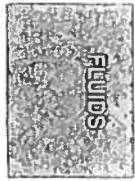


illustration	description	C_d	C_v	C_t
A	sharp-edged	0.62	0.63	0.98
B	round-edged	0.98	1.00	0.98
C	short tube (fluid separates from walls)	0.61	1.00	0.61
D	short tube (no separation)	0.82	1.00	0.82
E	short tube with rounded entrance	0.97	0.99	0.98
F	reentrant tube, length less than one-half of pipe diameter	0.54	0.55	0.99
G	reentrant tube, length 2 to 3 pipe diameters	0.72	1.00	0.72
not shown	smooth, well-tapered nozzle	0.98	0.99	0.99



An expression for the tank area, A_t , as a function of h , must be determined. Then, the time to empty the tank from height h_1 to lower height h_2 is

$$t = \int_{h_1}^{h_2} \frac{A_t dh}{C_d A_o \sqrt{2gh}} \quad 3.99$$

For a tank being fed at a rate, $\dot{V}_{in}$, which is less than the discharge through the orifice, the time to empty expression is

$$t = \int_{h_1}^{h_2} \frac{A_t dh}{(C_d A_o \sqrt{2gh}) - \dot{V}_{in}} \quad 3.100$$

When a tank is being fed at a rate greater than the discharge, equation 3.100 will become positive, indicating a rising head. t then will be the time it takes to raise the fluid level from h_1 to h_2 .

The preceding discussion has assumed that the tank has been open or vented to the atmosphere. If the fluid is discharging from a pressurized tank, the total head will be increased by the gage pressure converted to head of fluid by means of equation 3.52.

Example 3.22

A 15' diameter tank discharges 150°F water through a sharp edged 1" diameter orifice. If the original water depth is 12' and the tank is continually pressurized to 50 psig, find the time to empty the tank.

At 150°F, $\rho = 61.20 \text{ lbm/ft}^3$

For the orifice,

$$A_t = 0.00545 \text{ ft}^2, \quad C_d = 0.62$$

$$h_1 = 12 + \frac{(50)(144)}{61.2} = 129.65 \text{ ft}$$

$$h_2 = \frac{(50)(144)}{61.20} = 117.65 \text{ ft}$$

From equation 3.97,

$$t = \frac{2[\pi(7.5)^2](\sqrt{129.65} - \sqrt{117.65})}{(0.62)(0.00545)\sqrt{(2)(32.2)}} = 7035 \text{ seconds}$$

6 CULVERTS AND SIPHONS

A culvert is a water path used to drain runoff from a obstructing geographical feature. Most culvert design are empirical. However, if the entrance and exit of the culvert both are submerged, the discharge will be independent of the barrel slope. (Chapter 5 treats this subject in greater detail.) In that case, equation 3.1C can be used to evaluate the discharge. h is the difference in surface levels of the headwater and tailwater.

$$\dot{V} = C_d A \sqrt{2gh} \quad 3.1C$$

PHOTOGRAPHER AND DATE OF PHOTOS DANIEL A NELSON 5/21/2012

FILE NUMBER 74-15845



The above photo shows the headgate.



This is a photo of the fish screen. The orifice is located in the box to the left of the roller screen. Water comes in from the ditch behind the photographer, then goes through the screen, and out the ditch through the paddle wheel.