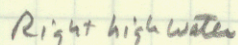
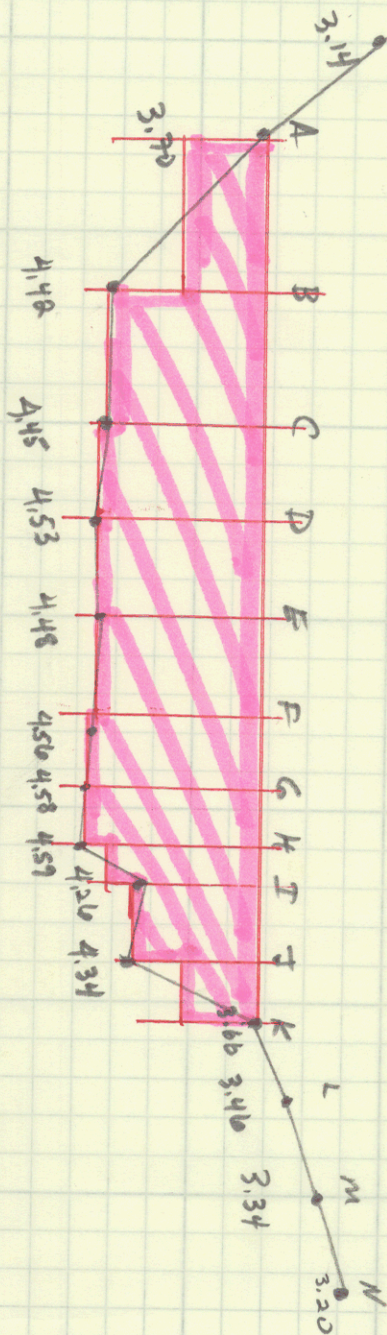


(looking downstream)



Right Bank Full
(downhill side)



**Water Resources
Field Services
208 663 4667**

Mannings equation calculations for A25-04152A and other Posey Creek Ditch rights.

Ditch Profile

Station	Horizontal	Rod	Wtr Surf	Depth
Left Bank	8.0	3.14	3.66	0.00
A	7.5	3.70	3.66	0.04
B	6.7	4.42	3.66	0.76
C	6.0	4.45	3.66	0.79
D	5.5	4.53	3.66	0.87
E	5.0	4.48	3.66	0.82
F	4.5	4.58	3.66	0.90
G	4.1	4.58	3.66	0.92
H	3.8	4.58	3.66	0.93
I	3.6	4.28	3.66	0.60
J	3.2	4.34	3.66	0.68
K	2.9	3.66	3.66	0.00
L	2.5	3.48	3.66	0.00
M	2.0	3.34	3.66	0.00
N	1.5	3.20	3.66	0.00

Wetted Portion

Section	Width	Depth	Area	Wetted P.
AB	0.8	0.40	0.320	1.08
BC	0.7	0.78	0.543	0.70
CD	0.5	0.83	0.415	0.51
DE	0.5	0.85	0.423	0.50
EF	0.5	0.86	0.430	0.51
FG	0.4	0.91	0.364	0.40
GH	0.3	0.83	0.278	0.30
HI	0.2	0.77	0.153	0.39
IJ	0.4	0.64	0.256	0.41
JK	0.3	0.34	0.102	0.74
Sum			3.283	5.530

Hydraulic Radius = Area/WP = 0.594 ft

Mannings n = 0.045 Perry, Civil Engineering, 1959. "Rock, jagged, 0.040 - 0.045"

Slope

Upstream rod 3.08
Downstream rod 3.40

Change in elevation 0.32

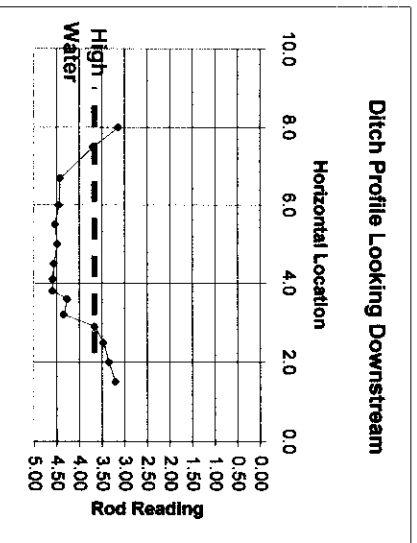
Distance 21

Slope 0.015238

Mannings Equation: $Q = (1.486/n) * Area * (Hydraulic\ Radius)^{2/3} * (Slope)^{1/2}$

USBOR Water Measurement Manual p 170

Q = 9.5 cfs



Wetted Perimeter calculated using distance formula:

square root ((width1 - width2)² + (depth1 - depth2)²)